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# 高性能混凝土结构火安全可恢复性研究进展

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**摘要:**从材料、构件、结构3个层面梳理了高性能混凝土结构火安全可恢复性的相关研究文献,厘清发展脉络。分析结果表明:基于混凝土结构火安全可靠度要求,可以通过建筑设计、修复与加固等流程与措施,使得高性能混凝土结构火灾后的性能恢复到初始水平甚至有所提升;利用修复剂涂抹或浸泡、水养护的方式均能实现火灾后高性能混凝土强度的逐渐恢复与裂缝愈合;利用电化学再碱化技术可实现火灾后混凝土pH值的恢复,减少碳化深度;利用碳纤维加固火灾后构件可实现承载力的恢复,但是对于刚度恢复程度较低;梁侧钢板加固、粘钢加固可实现火灾后构件承载力、刚度与延性的可恢复性。这些混凝土结构火安全可恢复性的方法可以为高性能混凝土结构的防灾减灾提供理论指导。

**关键词:**高性能混凝土结构;火灾损伤;修复加固;火安全可恢复性

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## Research Progress on Fire Safety Resilience of High Performance Concrete Structure

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**Abstract:** The relevant research literatures on fire safety resilience of high performance concrete structures were collected and classified as three levels i. e., materials, components and structures, and the development context of fire safety was clarified. The analysis results show that according to fire safety reliability requirements for concrete structures, the performance of structure after being exposed to fire can be recovered and even being improved compared to the initial level by means of structural design, reparation and strengthening. The strength of high performance concrete can be recovered gradually and the size of crack in the concrete surface can be reduced by brushing or soaking reparation agent and water curing. The pH value of concrete can be recovered and the carbonation depth can be decreased by the electrochemical realkalization technique. The load bearing capacity of concrete components after fire can be recovered by carbon fiber reinforced polymer, but the stiffness is recovered a little. The load bearing capacity, stiffness and ductility of concrete components after fire are resilient by the bolted side-plating technique and bonding steel plate. All these measures of fire safety resilience for high

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performance concrete structures can be used to guide the design of practical engineering for disaster prevention and reduction.

**Key words:** high performance concrete structure; fire damage; reparation and strengthening; fire safety resilience

## 0 引言

近些年来,随着中国高层建筑的大量兴建,高层建筑的高度越来越高,荷载也越来越大,为减小构件尺寸以获得更多的使用空间,高性能混凝土得到了广泛应用。以往的研究表明<sup>[1]</sup>,高性能混凝土在高温作用下比普通混凝土更易产生爆裂现象。爆裂会严重削弱构件的截面尺寸,使内部混凝土进一步暴露在火场中,甚至会使内部钢筋暴露,会大大降低构件的承载力及安全性。

同其他灾害损伤(如地震、腐蚀、材料老化)相比,混凝土结构无论是在火灾中的破坏形式,还是灾后构件的材料性能、受力状况都存在本质上的差异<sup>[2]</sup>。目前土木工程方面火灾可恢复性的提法已有研究涉及:Kodur<sup>[3]</sup>建议通过基于火安全的性能设计,克服火灾对建筑结构、基础设施工程的危害,提升城市结构的可恢复性;Selamet 等<sup>[4]</sup>、Fischer 等<sup>[5]</sup>分别对火灾与荷载耦合作用的组合楼板次梁与主梁连接、主梁与柱连接的力学性能进行数值模拟;Gerasimidis 等<sup>[6]</sup>研究了爆炸后引起的火灾对高层钢结构可恢复性的影响;Rinaudo 等<sup>[7]</sup>研究通过快速测定温度分布,尽快恢复隧道的服役能力。基于此,探讨高性能混凝土结构火安全可恢复性的理念:通过建筑设计、损伤监测与评估、修复与加固等流程和措施,混凝土结构的火灾(高温)后性能可以恢复到初始状态水平甚至有所提升。

经过近 60 年的发展,中国已经对高性能混凝土结构受火(高温)作用进行了系统性研究,形成了火安全设计、修复与加固的系列方法与理论。因此,有必要对已有的火安全可恢复性研究成果进行梳理与归类。Xiao 等<sup>[8-9]</sup>已对中国火(高温)作用的高性能混凝土材料力学性能、热工参数、爆裂影响因素等方面进行了总结与阐述,且本构关系、爆裂的预防措施分析也有涉及。在此基础上,本文通过对高性能混凝土结构火安全文献资料的收集,从材料、构件、结构 3 个层面对中国火安全可恢复性研究进行归类与分析,探求高性能混凝土结构火安全可恢复性的内容组成与实现方法,为实际工程的防灾减灾提供设计、修复、修补与加固等方面的理论支持与指导。

## 1 材料火灾损伤机理及可恢复性

### 1.1 受火损伤机理

混凝土受高温作用后水分要逐渐脱去,300~500℃时构件已完全脱水<sup>[10]</sup>。当温度达到 500℃时,通过热重分析可知,高强、自密实混凝土的孔隙率分别为 20.6%,18.5%<sup>[11]</sup>。在脱水的同时,柳献等<sup>[11]</sup>发现在 400~500℃时氢氧化钙的分解将造成大量孔隙。利用汞压力测孔法可得,高强混凝土在 500℃时的总孔隙率是 400℃时的 1.5 倍<sup>[11]</sup>,自密实混凝土总孔隙率在 450℃时可达到不受火试件的 1.8 倍<sup>[12]</sup>。600℃时孔的粗大化效应增加硬化水泥浆的孔隙<sup>[13]</sup>,Peng 等<sup>[14]</sup>认为该效应产生等效裂缝而降低高性能混凝土的强度。高性能混凝土微观结构在高温时的变化引起材料宏观力学性能的退化,降低结构安全性,因此提出材料火安全的可恢复性需求。

### 1.2 材料力学性能退化

在高温作用下,高性能混凝土结构材料强度变化情况见图 1<sup>[15-19]</sup>。由图 1 可以看出:高温下高性能混凝土抗压强度总体趋势大于高温后的相应值,高温后钢筋强度反而高于高温下的钢筋强度。具体为:高温(700℃)后钢筋强度不低于初始值的 90%,说明在火灾作用下钢筋强度具有可恢复性;高温(800℃)后混凝土抗压强度不到初始值的 20%,说明混凝土在火灾作用下强度显著降低,需要采取措施提升其灾后性能,确保结构火安全。

### 1.3 修复与材料性能火安全可恢复性

由图 1 分析可知,钢筋在高温后力学性能具有可恢复性,而高性能混凝土的性能劣化较严重,因此下面主要探讨高温后混凝土的性能修复研究。

#### 1.3.1 修复剂修复

修复剂修复是指利用修复剂渗入混凝土内部后激活水泥的水化效应,生成的 C-S-H 凝胶体填充混凝土空隙提高内部密实性,实现混凝土强度、抗渗性恢复的做法。吕云霞等<sup>[20]</sup>、李庆涛等<sup>[21-22]</sup>的研究表明:修复剂能大幅提高混凝土经受高温后的抗压强度并改善其抗渗性能,自然冷却的修复效果优于洒水冷却,修复剂采用聚氨酯,浸泡法比涂刷法的抗压

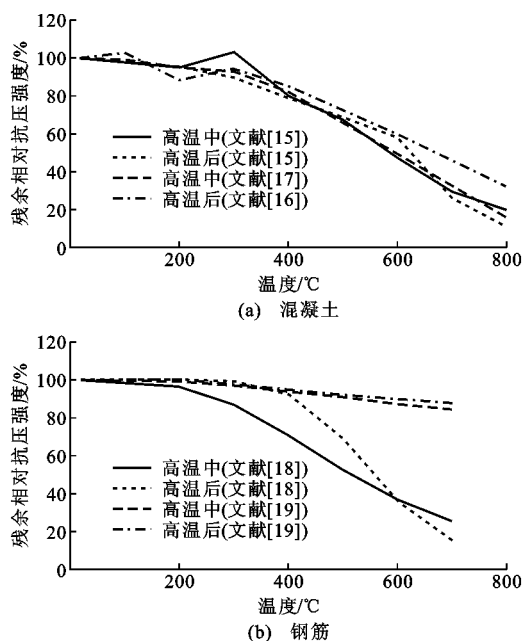


图 1 高性能混凝土、钢筋高温性能

Fig. 1 Performances of Concrete and Steel Bar Exposed to High Temperature

强度恢复程度高<sup>[20]</sup>;基于抗压强度恢复程度与经济性,建议涂刷法修复剂合理用量为  $0.3 \text{ kg} \cdot \text{m}^{-3}$ <sup>[21]</sup>;利用修复剂对高温( $550^\circ\text{C}$ )后钢筋与混凝土拉拔试件浸泡 12 h,修复试件的极限黏结强度高于未修复试件,而极限滑移却更小,可有效提高试件的黏结性能<sup>[22]</sup>。

### 1.3.2 再养护修复

再养护修复是指将受火(高温)后的试件自然、炉中、洒水或淬火冷却,然后在空气或水中或相当湿度的环境中进行养护,引发混凝土中水泥石的再水化,生成尺寸更小的胶凝产物(C-S-H)填充经历高温的混凝土孔隙,实现混凝土强度、裂缝宽度甚至碳化深度等的恢复行为。Poon 等<sup>[23]</sup>研究发现经历  $600, 800^\circ\text{C}$  的试件 56 d 水中养护后强度可恢复到初始值的 93%, 79%, 且部分试件的裂缝宽度最大为 0.31 mm, 满足 ACI 规范的要求。不但裂缝宽度变小, Li 等<sup>[24]</sup>研究发现裂缝长度也会变短, 且自然冷却试件的修复程度高于炉中冷却。经历的温度越低, 混凝土强度恢复程度越高<sup>[23-25]</sup>, Li 等<sup>[25]</sup>还发现试件经历温度低于  $450^\circ\text{C}$  时混凝土碳化深度被修复, 但  $750^\circ\text{C}$  时混凝土碳化深度修复不起作用, 另外洒水冷却比自然冷却的碳化深度更大。不超过  $800^\circ\text{C}$  时洒水冷却<sup>[25-26]</sup>、 $800^\circ\text{C}$  后炉中冷却<sup>[24]</sup>的混凝土以及  $800^\circ\text{C}$  后淬火冷却<sup>[27-29]</sup>的水泥基复合材料的强度恢复程度高于自然冷却, 但 Chen 等<sup>[26]</sup>发现当温度

为  $1000^\circ\text{C}$  时恢复效果相反。Chen 等<sup>[26]</sup>还发现温度低于  $800^\circ\text{C}$ 、龄期为 3 d 的试件强度恢复性能最好,  $1000^\circ\text{C}$  时 28 d 的试件强度恢复性能最好。

### 1.3.3 电化学再碱化修复

电化学再碱化修复是指将钢筋作为阴极、外部电极作为阳极通以直流电, 在电场、渗透、毛细管虹吸等作用下高碱性电解质溶液渗入混凝土内部, 钢筋表面发生电化学反应产生的  $\text{OH}^-$  部分滞留在钢筋周围, 部分向混凝土表面渗透, 从而改善混凝土微观结构并提升混凝土保护层 pH 值( $\text{pH} > 11.5$ <sup>[30]</sup>)的做法。邹云华等<sup>[31]</sup>的研究表明: 高温( $800^\circ\text{C}$ )后 7 d 混凝土中性化深度由 8 mm 变成 0 mm, 建议电流  $I \leq 1 \text{ A} \cdot \text{m}^{-2}$  及选择  $\text{Na}_2\text{SiO}_3$ ,  $\text{Na}_2\text{CO}_3$  作为电解质。熊焱等<sup>[32]</sup>研究高温( $600^\circ\text{C}$ )后混凝土修复(最长 21 d)得出: 高温分解后游离的 f-CaO 与  $\text{CO}_3^{2-}$  结合生成  $\text{CaCO}_3$  填充混凝土内部孔隙, 实现混凝土抗压强度、耐久性的恢复。

由此可见, 修复剂修复、电化学再碱化修复、再养护修复对于经历最高温度  $550, 800, 1000^\circ\text{C}$  的混凝土材料可实现火安全可恢复性, 3 种修复手段均能实现抗压强度的可恢复, 尤其是修复剂修复对抗渗性的可恢复、再养护修复对裂缝宽度的可恢复、电化学再碱化修复对碳化深度的可恢复有独到的优势。

## 2 构件火灾损伤及可恢复性

### 2.1 火荷载密度

建筑中可燃材料的数量称为火灾荷载, 为了消除建筑面积因素对火灾荷载的影响, 引入火荷载密度, 即房间内所有可燃材料完全燃烧时所产生的总热量与房间的特征参考面积之比<sup>[33]</sup>。火荷载密度的随机性导致结构构件温度场不确定, 最终影响结构火安全的可靠性: Xiao 等<sup>[34]</sup>研究发现, 随机性造成高强混凝土柱的可靠指标显著降低。Xie 等<sup>[35]</sup>基于贝叶斯理论对高层建筑中火荷载密度数据进行分析, 建立高层办公楼、住宅的火荷载密度计算模型, 并建议适用的最大面积分别为  $120, 35 \text{ m}^2$ 。火荷载密度的确定为高性能混凝土结构火安全设计奠定了基础。

### 2.2 火灾损伤与爆裂

Chan 等<sup>[13]</sup>研究发现高温下高性能混凝土与普通混凝土均会产生剥落, 使得试件强度下降, 造成结构性损伤。剥落的形式包括集料撕裂、表层剥落、角部开裂剥离、爆裂等<sup>[36]</sup>, 其中爆裂特别危险, 会造成

灾难性后果<sup>[36-37]</sup>,爆裂产生的混凝土碎片给逃生人员与救火队员带来危险<sup>[38]</sup>,且给结构造成安全风险<sup>[39]</sup>。

### 2.2.1 爆裂机理

当前,爆裂机理尚未清楚,知名爆裂机理为蒸汽压论<sup>[40]</sup>、热应力论<sup>[41]</sup>以及二者的结合<sup>[42]</sup>。中国学者基于爆裂特征,运用试验研究与理论分析相结合的方式,对爆裂的机理进行相关探讨。

(1)热开裂论:傅宇方等<sup>[43]</sup>认为开裂不仅降低混凝土强度,也提高其渗透性能,基于此建立热开裂论:裂缝作为通道可以释放孔隙水(汽)压力,在快速升温过程中,裂缝数量和孔隙水(汽)压力均会出现增加趋势,当裂缝增加所提供的空间足以缓解孔隙水(汽)压力时,混凝土爆裂发生的可能性就降低,否则反之;裂缝的累积与水化物的分解不断降低混凝土的强度;因此,热开裂与孔隙水(汽)压力的相互作用是混凝土发生爆裂的根本原因。该学说解释了爆裂发生的随机性及缓慢升温也会发生爆裂的现象。

(2)蒸汽爆炸模型:高宇剑<sup>[44]</sup>利用压力积聚理论中孔隙压力与混凝土抗拉劈裂强度的大小关系来解释爆裂的“裂”,利用“沸腾液体膨胀蒸汽爆炸”理论中高温下混凝土内部水的气相、液相在混凝土裂缝处体积急剧变化带来的爆炸声并将混凝土碎块抛射出去来解释爆裂的“爆”,二者相结合即为高强混凝土发生爆裂的机理。此模型还可以解释普通混凝土发生爆裂的现象。

爆裂机理的不确定性源自爆裂的随机性。爆裂随机性的体现:首先是爆裂的温度,Chan等<sup>[13]</sup>的试验结果为400℃或500℃,Poon等<sup>[45]</sup>的试验结果为400~600℃;其次是爆裂深度,Xiao等<sup>[46]</sup>通过高性能混凝土剪力墙火灾试验发现爆裂的深度最深可以达到60mm以上;还有爆裂的位置,Ichikawa等<sup>[47]</sup>数值模拟混凝土墙高温性能认为爆裂位置距离受火面10mm,Gao等<sup>[48]</sup>运用ABAQUS程序进行梁的受火分析,建议用压应力范围来预测爆裂位置。爆裂的随机性影响遭遇火灾(高温)作用的混凝土结构构件温度场,引起力学性能退化的不确定性,不利于混凝土结构火安全可恢复性的实现,有必要采取抑制爆裂的措施。

### 2.2.2 爆裂抑制

降低爆裂风险最有效的措施就是在混凝土受火面外包热障碍和(或)在混凝土中掺入聚丙烯纤维<sup>[39]</sup>。基于此,根据中国学者对于爆裂的抑制研究情况,分成外包型与聚丙烯纤维来进行阐述。

(1)聚丙烯纤维(Polypropylene Fiber, PPF)。

PPF掺入高强混凝土中抑制爆裂的机理:利用PPF熔点低在高温中熔化增加毛细孔<sup>[49]</sup>,熔化的PPF被周围水泥胶体所吸收而给液态水、蒸汽与气体带来新的传输路径<sup>[12,50]</sup>,使得热量与蒸汽可以从水泥胶体内部逸出,避免混凝土高温下进一步损伤,实现减轻或抑制爆裂。根据陈明阳等<sup>[51]</sup>建立的PPF体积掺量与混凝土强度的关系表达式以及收集的关于抑制爆裂的掺量研究,绘制成图2<sup>[11,16,51-58]</sup>。从图2可以看出,绝大部分试验数据位于直线的下方,表明该表达式(图2中直线)可作为PPF掺量上限值。PPF掺入高性能混凝土抑制爆裂的措施可在建筑火安全设计阶段采纳。

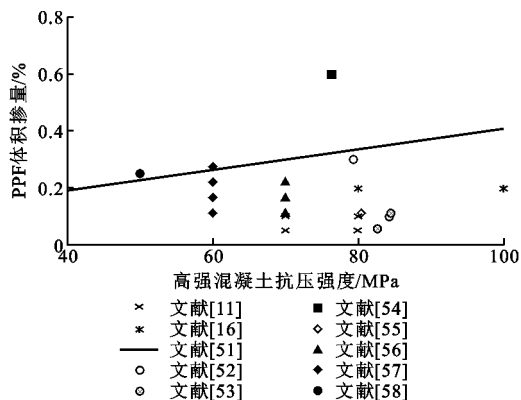


图2 无爆裂的PPF体积掺量与高强混凝土抗压强度关系

Fig. 2 Relationship Between Volume Ratio of PPF and Compressive Strength of HSC Without Explosive Spalling

(2)外包型。外包型是指通过在构件表面涂刷、安装1层(或多层)热惰性材料,延缓热量往构件内部传输,降低构件的受热温度,使构件在火灾中得到隔热保护。钱春香等<sup>[59]</sup>研究表明:采用防火涂层(厚6mm左右)可以在一定程度上抑制高性能混凝土爆裂的发生,建议选择含有氮磷碳化学阻燃成分的厚涂型防火涂料。Xiao等<sup>[46]</sup>进行高性能混凝土剪力墙的受火试验,研究发现带有防火牺牲层的剪力墙减轻爆裂面积60%以上。由此可以看出,外包型爆裂抑制措施适用于建筑火安全设计阶段及未遭受火灾作用的既有建筑。

### 2.3 修补与构件火安全可恢复性

对于既有建筑遭遇火灾作用时,爆裂对结构构件造成损伤影响最大的是爆裂导致混凝土脱落的深度。根据中国学者研究成果,采用如下措施来进行修补,可实现构件的火安全可恢复性。

#### 2.3.1 喷射混凝土

喷射修补是指将火灾(高温)后的烧伤混凝土保护层凿除,再用细石混凝土(钢细微混凝土)修补构

件表面的技术,修复后墙面平整、梁柱棱角清晰美观<sup>[60-61]</sup>。程良奎等<sup>[60]</sup>、史永忠等<sup>[61]</sup>应用该技术对火灾后建筑进行修复后认为这是一项技术先进、经济合理、技术可靠、行之有效的方法,并形成了损伤鉴定、设计、施工的完整流程。此外,张泽江等<sup>[62]</sup>介绍了喷射纤维防火保护材料,本文建议可用于高性能混凝土结构面层,不但可以进行面层修补,还可以起到二次抗火功能。

2.3.2 增大截面法

增大截面法是应用填充材料与钢筋对火灾损伤构件截面外侧进行包裹,增大原有构件截面尺寸实现构件修补与性能恢复提升的做法。刘利先等<sup>[63]</sup>采用普通混凝土作为增大截面法材料加固高温损伤柱,发现承载力比未加固受火试件显著提高,但明显低于未受火构件。胡克旭等<sup>[64]</sup>、韩重庆等<sup>[65]</sup>分别采用织物增强混凝土(TRC)、钢筋网细石混凝土加固火灾后(最大受火时间 60 min)预应力空心板,试验研究表明,加固试件发生弯曲破坏,其承载力、后期刚度恢复并超过未受火试件。

3 结构火安全及可恢复性设计

3.1 耐火极限

耐火极限作为混凝土结构火安全可恢复性的重要指标,其计算方法已经较为系统。耐火极限的确定既可在火安全设计时采取措施提高结构可恢复性,也可用于指导火灾后结构修复与加固。将混凝土结构板、梁、柱耐火极限的计算方法列于表 1<sup>[66-73]</sup>。由表 1 可以看出,非加固板的耐火极限计算方法尚未有报道。Zhang 等<sup>[74]</sup>基于能量原理<sup>[75]</sup>求得了承载力,并通过计算值等于实测值得到了非加固板耐火极限的具体值,韩重庆等<sup>[76]</sup>通过预应力空心楼板的试验研究发现,约束板、简支板的耐火极限

与荷载水平呈幂函数关系。郑永乾等<sup>[77-78]</sup>利用 ABAQUS 模拟研究高性能混凝土、普通混凝土剪力墙耐火极限,结果表明:轴压比、高厚比、墙厚、侧向荷载比影响程度显著,与耐火极限均呈非线性关系。陆洲导等<sup>[79]</sup>通过单层单跨无黏结预应力框架结构受火试验认为应尽量选用耐火性能好的材料并采取合理的措施来减小火灾对温度的影响,实现结构耐火极限的提升。

3.2 残余承载力

残余承载力的计算可以指导灾后的结构加固,为增强结构可恢复性提供依据。根据承载力退化程度的不同,选择不同的加固方案。残余承载力是钢筋混凝土结构火安全可恢复性的最重要指标,高性能混凝土结构构件承载力计算方法可采用等效截面法。

等效截面法按照以下流程建立<sup>[49]</sup>:首先根据构件截面的温度分布确定等温线的位置,再按照截面承载力等效原则将各温度区段内的截面宽度按照该区域混凝土温度下强度与常温强度的比值对截面宽度进行折减,得到相应的等效截面,最后根据力学平衡建立构件承载力的计算方法。由图 3 可以看出,计算值与实测值分布在象限中分线的两侧<sup>[49,80]</sup>,表明二者吻合较好,对于远离的数据点(6.604, 4.919),可能是因为发生爆裂的原因造成数值落于象限中分线的下方。

3.3 加固与火安全可恢复性

3.3.1 钢加固法

钢加固法是指将火灾(高温)后的钢筋混凝土构件如钢板、角钢等与混凝土相结合进行加固,达到恢复甚至提升构件承载力的方法,包括梁侧锚钢加固、粘钢加固、包钢加固等方式。Jiang 等<sup>[81]</sup>研究火灾后梁侧锚钢加固梁(Bolted Side-Plating, BSP)的受

表 1 构件耐火极限的计算方法

Tab. 1 Calculation Methods for Fire Resistance of Specimens

构件类型		计算方法	公式来源	加固材料
板	加固	$R_f = S_h S_{t_f} S_{\beta} S_{\rho} S_{t_{cov}} S_m$	文献[66]	CFRP
梁	加固	$R(\gamma, c, \rho, \frac{1}{d}, \frac{A_{sc}}{A_{st}}, \mu_{ag}, b) = \varphi(\gamma) \omega(c, \rho_s) \psi(\frac{1}{d}, \rho_s) \xi(\frac{A_{sc}}{A_{st}}) \mu_{ag} \phi(b)$	文献[67]	FRP
		$R(\gamma, c, \rho, \frac{1}{d}, \frac{A_{sc}}{A_{st}}, \mu_{ag}, b) = \varphi(\gamma) \omega(c) \psi(\frac{1}{d}, \rho_s) \xi(\frac{A_{sc}}{A_{st}}) \mu_{ag} \phi(b)$	文献[68]	FRP
	非加固	$R = (107.97\rho + 0.98)(2.12c + 70.02)/(10.89m^2 - 7.91m + 3.82)$	文献[69]	
		$t_{max,1} = 35.994(P/P_u)^{-1.327}, t_{max,2} = 33.739(P/P_u)^{-1.434}$	文献[70]	
柱	加固	$R_f = \beta_n \beta_p / \beta_n$	文献[71]	防火涂料
	非加固	$R_f = \beta_p \beta_l \beta_b \beta_{hdb} \beta_e \beta_p \lambda_1 \lambda_2 \lambda_3 \lambda_4 \lambda_5$	文献[72]	
		$t = 190 - 550w$	文献[73]	

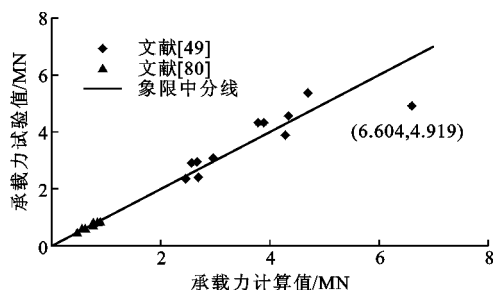


图3 等效截面法的应用

Fig. 3 Applications of Equivalent Cross-section Method

剪性能,结果表明抗剪承载力、刚度、延性都得到了恢复与提升,恢复程度随钢板高度增加、螺栓间距减小而增大。对于试验中观察到的受压区局部屈曲问题<sup>[81]</sup>,姜常玖等<sup>[82]</sup>发现设置加劲肋可有效限制局部屈曲,还可采取增加钢板厚度或减小螺栓距离等措施<sup>[83]</sup>。梁侧下部锚固钢板简支梁<sup>[82]</sup>、梁顶部与底部同时粘贴钢板<sup>[84]</sup>或仅底部粘贴角钢<sup>[85]</sup>的连续梁虽能实现承载力恢复但是也带来延性降低问题,韩重庆等<sup>[84]</sup>研究发现仅在梁底部粘贴钢板则能同时实现承载力与延性系数的恢复。基于粘钢法加固承载力恢复程度随着温度增大逐渐下降,宋永娟等<sup>[86]</sup>建议粘钢法适用试件经历最高温度不超过 550 ℃。包钢加固的具体措施为 U 形钢板加固高温后梁<sup>[87]</sup>、薄壁钢管加固高温后柱<sup>[88]</sup>、角钢加固火灾后柱<sup>[89-90]</sup>等,结果表明梁柱承载力均能实现恢复并提升,“薄壁钢管+钢管根部焊接角钢+螺栓锚固”<sup>[88]</sup>加固高温后柱还能实现刚度恢复与延性系数提升。

### 3.3.2 纤维加固法

碳纤维增强复合材料(Carbon Fiber Reinforced Polymer, CFRP)广泛应用于钢筋混凝土板、梁、柱等试件的受火(高温)加固。火灾后加固试验发现<sup>[64,91-98]</sup>,采用底部粘贴 CFRP 加固的板、底部粘贴 CFRP 或不等间距 U 形 CFRP 箍加固的梁、外包 CFRP 加固的柱试件与框架结构的承载力能够恢复至常温水平,但刚度恢复能力较差,在板<sup>[64,92-93]</sup>、梁<sup>[94-95]</sup>、柱<sup>[91,96]</sup>、框架<sup>[97-98]</sup>等研究中均有体现。徐玉野等<sup>[99]</sup>研究发现采取 CFRP 等间距环箍包裹加固矩形梁、U 形包裹加压条的方式加固 T 形梁均实现了承载力的恢复且有所提升外,试件弯曲刚度可恢复到未受火试件的水平且刚度提高水平随 CFRP 用量而增大。此外,陆洲导等<sup>[98]</sup>认为对于承载力与刚度要求较高时,可采用体外预应力法加固火灾后框架结构,但是要防止其脆性破坏。

## 4 结 语

(1)修复剂、水养护、电化学再碱化等方法可以实现混凝土材料火安全可恢复性。喷射混凝土、增大截面法可以修补高性能混凝土构件因为爆裂等原因造成的损伤,且增大截面法可实现构件火安全可恢复性。CFRP 加固能实现梁、板、柱的火安全可恢复性,BSP、粘钢加固可实现梁的火安全可恢复性,包钢加固中的“薄壁钢管+钢管根部焊接角钢+螺栓锚固”可实现柱的火安全可恢复性。这些措施共同实现高性能混凝土结构体系的火安全可恢复性。

(2)PPF 仍然是目前高性能混凝土抑制爆裂的最优方式,对于防火涂料需要加强保护,避免局部破损,对于构件外挂防火板的做法则需对拆装工艺的可操作性进一步开展研究。

(3)形成了包括加固钢筋混凝土板、柱、梁的耐火极限计算方法,但未加固板、剪力墙、结构的耐火极限计算方法还需进行更多的试验或模拟分析研究。

(4)CFRP 是目前应用最广泛的火灾后混凝土结构加固材料,形成了从构件到结构的高温损伤混凝土框架结构体系加固研究,但对加固后如何防止构件或结构的火灾损伤还需要更多研究。

(5)等效截面法条理清晰,适用于高性能混凝土结构火灾(高温)或承载力计算。

(6)为实现高性能混凝土结构火安全可恢复,修复加固措施的实施工艺与标准还有待进一步完善。

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